



### Assessment of potential hurricane-induced shallow landslide sources for Naranjito Municipality, Puerto Rico Rex L. Baum<sup>1</sup>, Dianne L. Brien<sup>2</sup>,

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# Abstract

Here we describe the landslide initiation (source software, each sampled over its respective area) part of a landslide susceptibility assessment of Naranjito Municipality, Puerto Rico. Determining landslide initiation potential is part of an effort to determine debris-flow runout hazard for areas of Puerto Rico affected by Hurricane Maria. A nonlinear area and slope dependent (NASD) soil depth model and the Transient Rainfall Infiltration and Grid-Based Regional Slope-Stability Analysis (TRIGRS) applied across a high-resolution digital landscape show where shallow landslides are likely to initiate during future hurricanes. Soil texture and strength data aided model calibration against detailed event mapping. Slope-stability analysis varying cohesion and friction defined valid ranges of these soil strength parameters for landslide source slope and depth distributions derived from mapping and field studies. Subdividing the map area into geologically based parameter zones distinguished areas where average soil depth or category, with > 48 scarp points/km<sup>2</sup>, covers strength differed from other map units (granitic bedrock or karstic limestone, respectively). Statistical comparison of results from 9 different runout zones for highly mobile Hurricane Maria soil models coded in the REGOLITH soil-depth

parameter space, yielded a best fit to fieldmeasured soil depth in landslide scars. Full saturation with the water table at the ground surface and (topographic) slope-parallel flow embodied the most likely hydrologic conditions for landslide initiation. We refined the extent of potential shallow landslide source areas from TRIGRS using a simplified 3-D slope stability analysis, which eliminated many false positives and narrowed the factor of safety, FS, ranges containing 75% and 90% of scarp points. Receiver Operator Characteristics (ROC) analysis of computed FS ranked the predictive success in identifying head-scarp points. The True Positive Rate for FS=1 was reasonable using the chosen soil strength parameters. Susceptibility categories capture 75% (FS=1.12) and 90% (FS=1.22) of the points, with FS  $\leq$ 1.12, high;  $1.12 < FS \le 1.22$ , medium, and 1.22< FS, low susceptibility. The high susceptibility ~30% of the area. The source area susceptibility results provide the input for assessment of landslides.

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Figure 1. Location of study area overlain on geologic terrane map (Bawiec, 1998).

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## 1. Study Area

Puerto Rico lies at the east end of the Greater Antilles and is characterized by rugged topography. The study area and calibration areas lie in the east-west-trending Cordillera Central range (Fig. 1). Heavily faulted basement rocks, consisting mainly of oceanic crust, and volcaniclastic and intrusive rocks, underlie the range (Jolly et al.

Bawiec (1998) generalized the geology of Puerto Rico into 12 geologic terranes having related rock types (Fig. 1). Bessette-Kirton et al. (2019a) identified calibration areas in three geologic terranes (igneous intrusive, volcaniclastic, and submarine basalt and chert) where high densities of debris flows occurred. High densities of Hurricane María-induced landslides also were correlated to high antecedent soil moisture (Bessette-Kirton, et al. 2019b).

### 2. Field study

Observations and measurements of the size, shape, geologic materials, topographic setting, and other characteristics of landslide and debris-flow sources were collected in field studies (Baum et al. 2018, Bessette-Kirton et al. 2019b) (Fig. 2).



Figure 2. Source areas of shallow landslides in volcaniclastic terrane (left) and igneous intrusive terrane (rigt) (C. Cerovski-Darriau, USGS).



Figure 3. Factor of safety results for combination of cohesion, c', angle of internal friction,  $\phi'$ , and pore-pressure head, where *m* is the ratio of pore-pressure head to soil depth. Observed slope angle and depth at mapped landslide sources in various geologic terranes, green "x" granitoid intrusive; violet "+" submarine basalt; brown triangle, volcaniclastic. Factor of safety, FS, at slope and depth combinations observed at landslide sources indicates model success (FS<1 if *m*=1) or failure (FS>1 if *m*=1). For the pair of c' and  $\phi'$ values shown, FS>1 for dry conditions (*m*=0) at about 97% of sources and FS>1 at 4% of sources for water table at the ground surface with flow parallel to the slope (*m*=1). These parameters, c' = 0.75 kPa and  $\phi' = 54^{\circ}$ , had an overall success rate of about 93% for all three terranes (after Baum 2021).

## 4. Calibration

and c'- $\phi$ ' combination for each terrane.  $FS_{m=0} \ge 1$  and  $FS_{m=1} < 1$ , Composite

> 25 30 35 40 45 50 55 60 Angle of Internal Friction for Effective Stress.  $\phi_{l}(\circ)$

We generated trial soil maps (Fig. 5) for simple empirical and process-based soil-depth models. The trial maps were compared against observed landslide depths to obtain the best fit for each model type (Tello 2020). Then computing factor of safety, FS-1D, for each best-fit soil map and better-performing range of c' and  $\phi'$  (Fig. 4) and comparing FS-1D with the locations of landslides using Receiver Operator Characteristics (ROC) analysis (Beguria, 2006), showed which of all the best-fit soil models and which c'- $\phi$ ' combination were the most accurate predictor of landslides for each terrane.



Figure 5. Trial soil maps for two of nine soil-depth models tested for the (triangluar red) calibration area (Fig. 1). (A) Best-fit version of the Modified Nonlinear Area and Slope-dependent (NASD) model (Pelletier & Rasmussen, 2009), (B) Linear area and slope dependent model based on wetness index (Ho et al. 2012). The NASD model (A) was ultimately chosen as the overall best-fitting soil-depth model for this terrane (volcaniclastic).



![](_page_0_Picture_28.jpeg)

Figure 6. Modeling steps depicted through close-up view of maps of (A) soil-depth based on modified non-linear area and slope dependent model (NASD), (B) pore-pressure head assuming slope-parallel flow with water table at the ground surface, (C) 1-D factor of safety from TRIGRS 2.1 (Alvioli & Baum 2016), (D) simplified 3-D factor of safety (Baum et al. 2012).

We analyzed many possible combinations of cohesion, c', and friction angle,  $\phi'$ , over the observed range of slope and depth of landslide sources (Fig. 3). Compiling the performance of every c'- $\phi'$  pair like that in Fig. 3 led to Fig. 4, which showed the better-performing ranges of c' and  $\phi'$  overall and for specific geologic terranes. These ranges were used in computing the factor of safety with trial soil-depth maps (Fig. 5) to find the best soil-depth map

![](_page_0_Figure_32.jpeg)

Figure 4. Composite fraction of landslide sources predicted correctly as a function of cohesion, *c*', and angle of internal friction,  $\phi'$ , for three major bedrock types in Puerto Rico. Each pixel summarizes the net result of a pair of analyses like that in Fig. 3. Yellow ovals show approximate ranges of most successful predictions for each terrane. Factor of safety (FS) for dry conditions is  $FS_{m=0}$ ; factor of safety for water table at ground surface with slope-parallel flow is FS<sub>m-1</sub>. Each grid cell represents the fraction  $(NFS_{m=0} - NFS_{m=1})/Nt$ , where  $NFS_{m=0}$  is the number of source areas for  $FS_{m=0} >=1$ ,  $NFS_{m=1}$  is the number of source areas for which  $FS_{m-1} >= 1$ , and Nt is the number of source areas in the bedrock type (after Baum 2021).

# 6. Results

After calibrating the soil-depth models and strength parameters to find the best fit, we computed soil depth, pore-pressure head and 1D factor of safety (FS-1D) for the entire study area. We also used pore-pressure head as input to compute simplified 3D factor of safety (FS-3D). Fig. 6 depicts close-up views of these four outputs and Fig. 7 depicts FS-3D for the entire study area, with a aquaredepicting the area of the close-up (Fig. 6). Fig. 8 depicts FS-3D with an overlay of scarp points (Hughes et al. 2019)

![](_page_0_Figure_37.jpeg)

![](_page_0_Figure_38.jpeg)

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![](_page_0_Picture_45.jpeg)

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Figure 7. Final map of factor of safety for simplified 3-D slope stability analysis. FS-3D < 1.12 encloses 75% of Hughes et al. (2019) scarp points, FS-3D < 1.22 encloses 90% and FS-3D < 1.30 encloses 95%. The map extends well bevond the limits of Naranjito Municipality so that all drainage basins crossing the Municipality boundaries are complete. This is to ensure that all potential source areas are accounted for in modeling debris-flow transport (Brien et al. 2021).

![](_page_0_Picture_50.jpeg)

![](_page_0_Picture_51.jpeg)

Base by U.S. Geological Survey (2018), NAD83(2011) / Puerto Rico and Virgin Is. - Projected.

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